RODEO VIEWING PLATFORMS 363 ORCHARD ST, WALLA WALLA, WA 99362 WALLA WALLA COUNTY

LEGEND

ACI	AMERICAN CONCRETE INSTITUTE	LLH
AFF	ABOVE FINISH FLOOR	LLV
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	LP
ALT	ALTERNATE	LSL
APA	AMERICAN PLYWOOD ASSOCIATION	LVL
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	MAX
AWC	AMERICAN WOOD COUNCIL	MFR
AWS	AMERICAN WELDING SOCIETY	MIN
BF#		MISC
		N
BOBD		
BODP		ND3
BOD	BOTTOM OF DECK	NFPA
BOF		NS
BP#	BASE PLATE NUMBER	NIS
С	CHANNEL	OPP
C/C	CENTER TO CENTER	OVS
CIP	CAST-IN-PLACE	PAF
CJ	CONSTRUCTION JOINT	PCF
CL	CENTER LINE	PERP
CLR	CLEAR	PL
COL	COLUMN	PLF
CONC	CONCRETE	PREFAB
CONT	CONTINUOUS	PSF
CSTC	CRUSHED SURFACING TOP COURSE	PSI
CTR	CENTER	PT
CP	CONTROL POINT	PVC
d.		R
		REE
DIAG		REQU
DL		SUJ
DN	DOWN	SIM
DSL	DRIFT SNOW LOAD	SL
DWL	DOWEL	SOG
(E)	EXISTING	STD
EA	EACH	SPEC
EF	EACH FACE	SQ
EJ	EXPANSION JOINT	SSL
EL OR ELEV	ELEVATION	T&B
EMBED	EMBEDMENT	TBD
EOD	EDGE OF DECK	TEMP
EQ SP	EQUALLY SPACED	TOC
ES	EXPOSED STRUCTURE	TOD
ESR	ENGINEERS SERVICE REPORT	TOF
EXP	EXPOSED	TOS
EW	EACH WAY	TOSW
FDN	FOUNDATION	TYP
FF	FINISH FLOOR	UNO
FOS		VERT
FOW/		W/
		VVP
FIG		VVUL
GLB	GLUE-LAMINATED BEAM	VVVVF
HP	HIGH POINT	
HSS	HOLLOW STRUCTURAL SECTION	
IBC	INTERNATIONAL BUILDING CODE	
ICC	INTERNATIONAL CODE COUNCIL	
INT	INTERIOR	- · · · · ·
JB	JOIST BEARING	
JST	JOIST	
К	KIP; KIPS	
KSI	KIPS PER SQUARE INCH	
KCJ	KEYED CONSTRUCTION JOINT	
L	ANGLE	
LB	POUND; POUNDS	
LF	LINEAR FOOT	
LL	DOUBLE ANGLE	

LONG LEG HORIZONTAL

LAMINATE STRAND LUMBER LAMINATE VENEER LUMBER

NATIONAL DESIGN SPECIFICATION

POWER ACTUATED FASTENER

POUNDS PER CUBIC FOOT

POUNDS PER LINEAR FOOT

POUNDS PER SQUARE FOOT

POUNDS PER SQUARE INCH

REINFORCEMENT; REINFORCING

POLYVINYL CHLORIDE

SAWN CONTROL JOINT

NORTHERN FOREST PRODUCTS ASSOCIATION

LONG LEG VERTICAL

LOW POINT

MAXIMUM

MINIMUM

NORTH

NEAR SIDE

OVERSIZE

PLATE

POINT

RADIUS

REFERENCE

REQUIRED

SIMILAR

SNOW LOAD

STANDARD

SQUARE

SLAB ON GRADE

SPECIFICATION

STAINLESS STEEL

TOP AND BOTTOM

TEMPORARY

TOP OF DECK

TOP OF STEEL

TYPICAL

VERTICAL WIDE; WIDTH

WIDE FLANGE

WORK POINT

WIND UPLIFT LOAD

WOOD COLUMN

STEEL COLUMN

WELDED WIRE FABRIC DECK SPAN DIRECTION

MOMENT FRAME OR BEAM

MOMENT CONNECTION NEW CONCRETE SLAB

TOP OF FOOTING

TO BE DETERMINED

TOP OF CONCRETE

TOP OF STEM WALL

UNLESS NOTED OTHERWISE

NOT TO SCALE

OPPOSITE HAND

PERPENDICULAR

PREFABRICATED

MANUFACTURER

MISCELLANEOUS



T001 T002 -	TITLE SHEET CODE AND LIFE SAFETY PLAN SURVEY
S001	GENERAL NOTES
S101	DEMOLITION PLAN
S111	OVERALL FOUNDATION PLAN
S112	FOUNDATION PLAN - PLATFORM A
5113	FOUNDATION PLAN - PLATFORM B AND C
5114	FOUNDATION PLAN - PLATFORM D
5115	
S121	
S122 S123	
S123 S124	
S124 S125	
S125	ELEVATED BOOTH ERAMING PLANS
S120	
S132	RAILING PLAN - PLATEORM A
S133	RAILING PLAN - PLATFORM B AND C
S134	RAILING PLAN - PLATFORM D
S135	RAILING PLAN - PLATFORM F
S201	ELEVATIONS
S202	ELEVATIONS
S301	PLATFORM SECTIONS
S401	STAIR PLANS AND PROFILE
S402	RAMP PLAN AND PROFILES
S501	SECTIONS AND DETAILS
S502	SECTIONS AND DETAILS
S511	SECTIONS AND DETAILS
S512	SECTIONS AND DETAILS
S513	SECTIONS AND DETAILS







2018 IBC - WASHINGTON ANALYSIS

602 – CONSTRUCTION CLASSIFICATION: 301-312 OCCUPANCY CLASSIFICATION: CHAPTER 4 – SPECIAL TYPES OF BUILDINGS: 508.3 - NON-SEPARATED OCCUPANCIES: 903 – AUTOMATIC SPRINKLER SYSTEM: 907 – FIRE ALARM AND DETECTION SYSTEM: TABLE 504.3 - ALLOWABLE BUILDING HEIGHT: PROPOSED BUILDING HEIGHT: TABLE 504.4 - ALLOWABLE NUMBER OF STORIES: PROPOSED NUMBER OF STORIES: TABLE 506.2 - TABULAR ALLOWABLE BUILDING AREA: 506.3 – FRONTAGE INCREASE: TOTAL ALLOWABLE AREA: ACTUAL BUILDING AREA: TABLE 601 – FIRE RESISTANCE RATINGS: TABLE 602 – FIRE RESITANCE RATING, EXTERIOR: 716 – OPENING PROTECTION: 1004 – CALCULATED OCCPANT LOAD: ACTUAL OCCUPANT LOAD: 1006 – EXITS REQUIRED/PROVIDED 1008 – MEANS OF EGRESS ILLUMINATION: 1010.1.10 – PANIC AND FIRE EXIT HARDWARE TABLE 1017.2 – EXIT ACCESS TRAVEL DISTANCE ALLOWABLE LONGEST EXIT TRAVEL DISTANCE: COMMON PATH OF TRAVEL: LONGEST COMMON PATH (BOOTH) 1105 – ACCESSIBLE ENTRANCES:

LEGEND - CODE ANALYSIS

OCCUPANT LOAD EXITING THE BUILDING OR FLOOR ABOVE

V-B A-5 N/A NONE NONE NONE 55' 34' UNLIMITED 2 UNLIMITED NOT USED UNLIMITED 31,530 SF 0-HOUR 0-HOUR NOT REQUIRED 1,571 1,059 4/7 >1 FOOTCANDLE NONE REQUIRED 200' 190'-6" 75' 60' 1

BUILDING:2018 INTERNATIONAL BUILDING CODE WITH WASHINGTON STATE AMENDMENTSELECTRICAL:2018 EDITION OF NATIONAL ELECTRIC CODEFIRE:2018 INTERNATIONAL FIRE CODEENERGY:2018 EDITIONS OF WA. STATE ENERGY CODEACCESSIBILITY:2012 ICC/ANSI A117.1

EXISTING CONDITIONS SURVEY

OF WALLA WALLA COUNTY FAIRGROUNDS RODEO ARENA JOB NO.: 21–588 DATA COLLECT: 12–27–2021 DRAWING DATE: 12–28–2021

LEGEND:

WV	
\bowtie	INDICATES WATER VALVE
^{cv} ⊠	INDICATES CONTROL VALVE
WSP S	INDICATES WATER STAND PIPE
TP	INDICATES TELEPHONE PEDESTAL
¢	INDICATES LIGHT POLE
Ē	INDICATES ELECTRIC PEDESTAL
T	INDICATES TRANSFORMER
{ 12	INDICATES DECIDUOUS TREE WITH TRUNK DIAMETER
•	INDICATES MONUMENT FOUND AS NOTED
\bigtriangleup	INDICATES MGS CONTROL POINT
	-INDICATES EDGE OF ASPHALT -INDICATES EDGE OF CONCRETE
	-INDICATES 5 FOOT INTERVAL CONTOUR
	-INDICATES 1 FOOT INTERVAL CONTOUR
_xx	-INDICATES FENCE LINE
-00	-INDICATES CHAINLINK FENCE LINE
	-INDICATES WOOD FENCE LINE

15 0

30

GENE	ERAL	NOTES		EAR	THWORK
1.	ALL (BUIL JURI	CONSTRUCTION SHALL BE IN ACCORDANCE WIT DING CODE (IBC) AND LOCAL RULES/STANDARDS SDICTION.	H THE 2018 EDITION OF THE INTERNATIONAL S OF GOVERNING AGENCIES HAVING	1.	REMOVE ALL VEGETATION AND COMPACT TOP 12" OF SUBGRADE UNDER FOOTINGS ⁻ 95% OF MODIFIED PROCTOR, ASTM D1557. IF BACKFILL IS REQUIRED UNDER NEW FOOTINGS OR SLABS PROVIDE 8" MAXIMUM LIFTS OF GW TO 95% OF MODIFIED PROC ASTM D1557
2.	THE DRA	CONTRACTOR SHALL PERFORM ALL WORK IN AC WINGS.	CCORDANCE WITH THE CONSTRUCTION	2.	PROVIDE 6" OF 5/8" MINUS CRUSHED ROCK UNDER ALL SLABS ON GRADE COMPACTE 95% OF MODIFIED PROCTOR ASTM D1557.
3.	SHO REVI DIME	P DRAWINGS AND PRODUCT DATA SHALL BE SUB EW AFTER THE CONTRACTOR HAS REVIEWED TI NSIONS, WEIGHTS OR GAUGES, FABRICATION P	BMITTED TO THE ENGINEER OF RECORD FOR HE DOCUMENTS FOR ACCURACY. QUANTITIES, ROCESSES, CONSTRUCTION METHODS,	3.	PRIOR TO DIGGING VERIFY LOCATION AND DEPTH OF UTILITIES AND OTHER UNDERGROUND INTERFERENCES. CALL TWO BUSINESS DAYS BEFORE YOU DIG AT 8
	COO THE	RDINATION OF THE WORK BETWEEN TRADES, OI SOLE RESPONSIBILITY OF THE CONTRACTOR. EI	R CONSTRUCTION SAFETY PRECAUTIONS ARE NGINEER OF RECORD REVIEW IS FOR GENERAL	FOU	NDATIONS
	DESI			1.	FOUNDATION DESIGN SHALL BE IN ACCORDANCE WITH IBC TABLE 1806.2
	SHO PURI THE	P DRAWINGS AND PRODUCT DATA DOCUMENTS POSE IS TO DEMONSTRATE THE WAY BY WHICH INFORMATION AND THE DESIGN CONCEPTS EXP	ARE NOT CONTRACT DOCUMENTS. THEIR THE CONTRACTOR PROPOSES TO CONFORM TO PRESSED IN THE CONTRACT DOCUMENTS.	2.	FOUNDATION DESIGN IS BASED ON THE ALLOWABLE SOIL PROPERTIES:SOIL BEARING2000 PSFCOEFFICIENT OF FRICTION0.35LATERAL BEARING DESSURE150 DSE/ET
•	FABF A.	RICATION, BUT NOT LIMITED TO: CONCRETE MIX DESIGN(S) IF fc IS OVER 2500 P	2SI	3.	GEOTECH REPORT PROVIDED BY MURAR ENGINEERING AND DESIGN.
	В. С.	CONCRETE REINFORCING AND CAST-IN-PLACE STRUCTURAL STEEL SHOP DRAWINGS	ANCHOR DRAWINGS IF fc IS OVER 2500 PSI	CON	ICRETE
	D. E.	WOOD TRUSS SHOP DRAWINGS AND CALCULA	TA TIONS	1.	STRUCTURAL CONCRETE COMPRESSIVE STRENGTHS AND AIR ENTRAINMENT SHALL
ð.	CON AND SHO THF	TRACTOR IS RESPONSIBLE FOR VERIFICATION C CONSTRUCTION CONDITIONS. CONTRACTOR SH P FABRICATION AND/OR FIELD ERECTION. WORK RESPONSIBILITY OF THE CONTRACTOR	DF SITE CONDITIONS, INSTALLATION STANDARDS HALL FIELD VERIFY ALL DIMENSIONS PRIOR TO COONE WITHOUT THE ENGINEERS APPROVAL IS		AS FOLLOWS: A. FOUNDATIONS - 2,500 PSI AND NO ADDED AIR ENTRAINMENT. B. EXTERIOR SLABS ON GRADE AND PEDESTALS - 4,500 PSI AND 6% AIR-ENTRAINM C. CONCRETE FLOOR DECK - 3000 PSI
	PRO THE	TECTION OF EXISTING STRUCTURES DURING TH RESPONSIBILITY OF THE GENERAL CONTRACTO	E COURSE OF THE CONSTRUCTION SHALL BE R.	2.	CAST IN PLACE CONCRETE SHALL MEET THE FOLLOWING REQUIREMENTS: ACI 117 - STANDARD SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS.
	ALL S AND TEST	SPECIAL INSPECTION AND TESTING SHALL BE PE TESTING AGENCY HIRED BY THE OWNER. CONT ING AGENCY FOR REQUIRED CONSTRUCTION IN	ERFORMED BY AN INDEPENDENT INSPECTION RACTOR TO COORDINATE WITH INSPECTION AND NSPECTIONS AND MATERIAL TESTING.		ACI 301 - SPECIFICATIONS FOR STRUCTURAL CONCRETE. ACI 302.1R - GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION. ACI 305R - HOT WEATHER CONCRETING. ACI 306 R- COLD WEATHER CONCRETING.
-	THE STAT	STRUCTURAL INTEGRITY OF THIS STRUCTURE IS E. WHILE UNDER CONSTRUCTION, ALL TEMPOR	S DESIGNED TO BE ATTAINED IN IT'S COMPLETED RARY BRACING AND/OR SHORING REQUIRED TO	3.	THE USE OF SUPER PLASTICIZERS AND WATER REDUCERS IS ALLOWED, BUT NOT REQUIRED. ALL ADMIXTURES SHALL BE CHLORIDE FREE.
_	MAIN	TAIN STABILITY PRIOR TO COMPLETION SHALL E JDING DESIGN AND INSTALLATION.	BE THE RESPONSIBILITY OF THE CONTRACTOR,	4.	UNLESS NOTED OTHERWISE, ALL CONCRETE FLAT WORK SHALL CONFORM TO THE FOLLOWING FINISHING TOLERANCES 1/8" GAP UNDER A 10'-0" STRAIGHT EDGE.
).	DESI	GN CRITERIA (PER 2018 IBC AND ASCE 7-16)	11	5.	WHERE CALLED FOR ON THE DRAWINGS, FLOOR SLABS ARE TO BE REINFORCED WIT
	14.1	/ERTICAL LOADS:	11		SHALL BE LAPPED TWO FULL MESHES OF FABRIC. WELDED WIRE FABRIC (WWF) SHA CONFORM TO ASTM A1064. Fy=60 KSI. WWW SHALL BE 6x6-W2.0x2.0.
	A.	DEAD LOADS: ROOF (FLAT)) PSF	6.	REINFORCING STEEL SHALL BE GRADE 60 DEFORMED BARS COMPLYING WITH ASTM SECTION A615. REINFORCING STEEL WHICH IS INDICATED ON THE PLANS AS BEING
	B.	FLOOR 5 LIVE LOADS (IBC 1607): ASSEMBLY AREAS 1	55 PSF		WELDED OR AS PART OF SPECIAL SHEAR WALLS OR SPECIAL MOMENT FRAMES SHAI COMPLY WITH ASTM A706 Fy=60 KSI, AND SHALL ALSO BE DEFORMED. WELDING OF REINFORCING BARS SHALL BE PER AWS D1.4.
		STAIRS / EXITS 1 OFFICES 5 POOF LIVE LOAD	100 PSF 50 PSF 20 PSE	7.	ALL DETAILING, FABRICATION AND PLACEMENT OF REINFORCING STEEL SHALL COMP
	C.	SNOW LOADS: GROUND SNOW LOAD F MINIMUM UNIFORM ROOF SNOW LOAD F	Pg=18 PSF Pm=30 PSF	8.	REINFORCEMENT LAP HOOKS, ETC.; SHALL BE PER THE REINFORCEMENT TABLE UNL NOTED OTHERWISE.
		SNOW EXPOSURE FACTOR:OSNOW LOAD IMPORTANCE FACTOR:Is	Ce=0.9 s=1.10	9.	THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR
		THERMAL FACTOR:CSLOPE FACTORCFLAT ROOF SNOW LOADF	Ct=1.2 Cs=1.0 Pf= 30 PSF		REINFORCEMENT: A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3" B. CONCRETE EXPOSED TO EARTH OR WEATHER: #6 THROUGH #18 BARS: 2"
	14.3	ATERAL LOADS:			#5 BAR, W31 OR D31 WIRE AND SMALLER: 1 1/2" C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
	A.	WIND DESIGN LOAD DATA: VELOCITY (3-SECGUST) EXPOSURE	V (ULT) = 110 MPH / V (ASD) = 85 MPH		SLABS, WALLS, JOISTS #14 AND 18 BARS: 1 1/2" #11 BAR AND SMALLER: 3/4"
		COMPONENTS AND CLADDING DESIGN WIND FO WALLS U 10 SF -	J.55 ORCES (IBC 1603.1.4) JLTIMATE 41.16 PSF		DEAMS, COLOMINS PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS: 1 1/2" D. REINFORCING SHALL EXTEND TO THE END OF THE CONCRETE AND MAINTAIN T COVER LISTED ABOVE AT THE ENDS.
		500 SF - ROOF ZONE 1 & 2e	-33.68 PSF -26.19 PSF	11.	ALL EXPOSED CORNERS OF CONCRETE SHALL BE FORMED INTO A 3/4" x 45 DEGREE CHAMFER, OR SCRIBED WITH A CONCAVE TOOLING DEVICE UNLESS NOTED OTHERW
	В.	SEISMIC DESIGN LOAD DATA: IMPORTANCE FACTOR	1.25	12.	ACCURATELY POSITION, SUPPORT AND SECURE REINFORCEMENT.
		RHO (N-S) 1 RHO (E-W) 1	1.3 1.3	13.	PROPORTION AND DESIGN MIXES TO RESULT IN CONCRETE SLUMP AT POINT OF
		MAPPED SPECTRAL RESPONSE ACCELERATION Ss = (S1 = (SEISMIC SITE CLASS	NS: 0.405 0.139		PLACEMENT 4" OR 8" FOR CONCRETE WITH VERIFIED SLUMP OF 2" TO 4" BEFORE ADD HIGH-RANGE WATER-REDUCING ADMIXTURE OR PLASTICIZING ADMIXTURE, PLUS OR MINUS 1 INCH. ADDITION OF WATER TO READY-MIX CONCRETE IN THE FIELD SHALL B ALLOWED ONLY IF ON TRIP TICKET AND RECORE DISCHARGE AND TESTING
		DESIGN SPECTRAL RESPONSE COEFFICIENT	2 TS: 0.398	14	DEPOSIT CONCRETE IN A CONTINUOUS OPERATION UNTIL THE PLACING OF CONCRET
		Sd1 = (SEISMIC DESIGN CATEGORY E	D.215 D	17.	COMPLETE. IF THE POUR IS TO BE DISCONTINUOUS, CONTRACTOR SHALL USE CONSTRUCTION JOINTS, AS DETAILED ON THE DRAWINGS.
		SEISMIC FORCE RESISTING SYSTEM AND RE ORDINARY CONCENTRICALLY BRACED FRAMES	ESPONSE MODIFICATION FACTOR:	15.	UNLESS NOTED OTHERWISE, REINFORCING IS NOT TO EXTEND THROUGH CONSTRUCTION JOINTS OF FLOOR SLABS-ON-GRADE.
		ORDINARY MOMENT FRAMES F LIGHT WOOD OR COLD-FORMED STEEL FRAMED BEARING WALLS WITH WOOD SHEAR PANELS OR	R = 4.5 (ANNOUNCER BOOTH FRAMING)	16.	REPAIR ALL SURFACE DEFECTS INCLUDING TIE HOLES, MINOR HONEYCOMBING AND OTHER VISUAL IRREGULARITIES WITH CEMENT MORTAR. MORTAR FOR PATCHING SH BE THE SAME COMPOSITION AS THAT USED IN THE CONCRETE. PATCHING SHALL BE
		STEEL SHEETS F	R = 6.5 (ANNOUNCER BOOTH)	4-	DONE AS SOON AS THE FORMS ARE REMOVED.
		SEISMIC RESPONSE COEFFICIENT Cs C DESIGN BASED SHEAR	D.153 D.153W	17.	RECESSES, OR SLAB THICKNESS CHANGES IN THE TOP 1/3 OF THE SLAB-ON-GRADE.
		ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PER ASCE 7-1	6, 12.8	18.	TO REDUCE SHRINKAGE CRACKS FIBER REINFORCING SHALL BE USED FOR SLABS O GRADE AND ELEVATED DECKS. FIBERS SHALL BE EITHER POLYPROPYLENE OR NYLC POLYPROPYLENE FIBERS SHALL BE MANUFACTURED BY THE FIBERMESH COMPANY, NYLON COMPANY, OR APPROVED EQUAL AND SHALL COMPLY WITH ASTM C1116, TYP NYLON FIBERS SHALL BE MANUFACTURED BY THE NYLON COMPANY, OR ENGINEER APPROVED EQUAL.

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DN. PE III. CONCRETE - POST INSTALLED ANCHORS

1. SPECIAL INSPECTIONS ARE REQUIRED PER IBC CHAPTER 17.

- ANCHORS SHALL BE INSTALLED IN CONCRETE THAT IS A MINIMUM OF 21 DAYS OR AS RECOMMENDED BY THE MANUFACTURER.
- 3. EXPANSION ANCHORS SHALL CONFORM TO THE FOLLOWING: A. HILTI KWIK BOLT-TZ2 EXPANSION ANCHORS INSTALL PER ESR-4266 OR DEWALT
 - POWER STUD +SD2 INSTALL PER ESR-2502 OF SIMPSON STRONG TIE BOLT 2 INSTALL PER ESR-3037. B. EXTERIOR ANCHORS SHALL BE STAINLESS STEEL OR GALVANIZED.
- SCREW ANCHORS SHALL CONFORM TO THE FOLLOWING: 5.
- A. SIMPSON STRONG TIE TITEN HD ANCHORS INSTALL PER ESR-2713 OR HILTI KH-EZ INSTALL PER ESR-3027
- OR DEWALT SCREW-BOLT+ INSTALL PER ESR-3889 B. ANCHORS EXPOSED TO WEATHER SHALL BE STAINLESS STEEL.
- ALL NAILING REQUIREMENTS LISTED ARE BASED UPON THE USE OF COMMON WIRE NAILS 5. (NOT SINKERS, BOX, ETC.) UNLESS NOTED OTHERWISE. ALTERNATIVE NAIL TYPES OF 6. SUBSTITUTION OF MANUFACTURED PRODUCT IS NOT PERMITTED UNLESS APPROVED BY EQUIVALENT DIAMETERS MAY BE SUBSTITUTED, WITH PRIOR APPROVAL OF THE THE ENGINEER OF RECORD IN WRITING. ENGINEER OF RECORD.

TIMBER

CAST-IN-PLACE ANCHORS

- 1. ANCHORS SHALL BE ASTM F1554 GRADE 55, UNLESS NOTED OTHERWISE.
- 2. FURNISH ANCHORS PREFABRICATED WITH DOUBLE HEAVY HEX NUTS JAMMED AT THE END EMBEDDED IN CONCRETE. WALL PLATE OR TRACK ANCHORS MAY BE L BOLTS WITH 6" MINIMUM EMBEDMENT.
- FURNISH WASHERS AND HEAVY HEX NUTS FOR SECURING THE BASE PLATE TO THE 3. ANCHORS.

4. NO HEATING OR BENDING OF THE ANCHORS IS PERMITTED. POSITION AND SECURE IN PLACE EXPANSION JOINT MATERIALS, ALL CAST IN PLACE ANCHORS AND OTHER EMBEDDED ITEMS BEFORE PLACING CONCRETE IN FORMS.

<u>STEEL</u>

- STRUCTURAL STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS:
- A. ALL W & WT SHAPES SHALL CONFORM TO A992 (50 KSI)
- B. ALL PLATES, CHANNELS, ANGLES, BARS, M SHAPES, S SHAPES, AND FLATS SHALL CONFORM TO A36 (36 KSI)
- C. ALL HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO A500, GRADE C (50 KSI) (46 FOR ROUND)
- 2. ALL DETAILING, FABRICATION, AND ERECTION OF STRUCTURAL STEEL SHALL COMPLY WITH THE REQUIREMENTS OF THE AISC 360-16 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS AND AISC 341-16 SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS FOR LATERAL FORCE RESISTING SYSTEM.
- ALL BOLTED CONNECTIONS SHALL BE MADE WITH HIGH STRENGTH BOLTS (A325) UNLESS 3. OTHERWISE NOTED ON THE DRAWING. BOLTED CONNECTIONS SHALL BE "SNUG TIGHT".
- 4. BOLT HOLES SHALL BE BOLT DIAMETER + 1/16". BOLT END AND EDGE DISTANCES AND BOLT LENGTHS SHALL BE PER AISC, UNLESS NOTED OTHERWISE. BASE PLATE BOLT HOLES MAY BE OVERSIZED BY 1/8".
- HOLES IN THE BASE MATERIAL SHALL NOT BE ENLARGED BY BURNING.
- ALL WELDING SHALL BE PERFORMED IN ACCORDANCE WITH THE SPECIFICATIONS AND 6. PROCEDURES OF THE AMERICAN WELDING SOCIETY BY AWS CERTIFIED WELDERS AND SHALL CONFORM TO AWS D1.1:2020. WELDERS PERFORMING THE WORK SHALL HAVE BEEN RE-TESTED WITHIN 6 MONTHS PRIOR TO THE START OF STEEL FABRICATION. WELDING FOR STRUCTURAL STEEL SHALL BE MADE WITH E70XX LOW HYDROGEN ELECTRODES. WELDING ON MAIN FORCE RESISTING SYSTEM AS INDICATED ON PLANS SHALL ALSO CONFORM TO AWS D1.8:2016 WITH WELDERS CERTIFIED TO AWS D1.8
- 7. FIELD WELDING SYMBOLS HAVE NOT NECESSARILY BEEN INDICATED ON THE DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE USE OF SHOP AND FIELD WELDING.
- ENDS OF HOLLOW STRUCTURAL SECTION COLUMNS AND EXPOSED MEMBERS SHALL 8. HAVE 3/16" CAP PLATES AND SEAL WELDS ALL ROUND.
- HOLES SHALL NOT BE CUT THROUGH BEAMS UNLESS INDICATED OR PRE-APPROVED BY THE ENGINEER OF RECORD IN WRITING.
- 10. ALL STRUCTURAL AND MISCELLANEOUS STEEL EXPOSED TO WEATHER SHALL BE HOT DIP GALVANIZED AFTER FABRICATION.
- 11. PRIME ALL FERROUS METALS FOR EXTERIOR EXPOSURE.
- 12. GROUT MATERIAL FOR BASE PLATES, SLEEVES, AND EMBEDDED STEEL SHALL BE NONMETALLIC, NON-SHRINK, PREPACKAGED GROUT CONFORMING TO ASTM C 1107.

METAL FLOOR DECK

- METAL FLOOR DECK BELOW CONCRETE SLABS SHALL BE ATTACHED TO ITSELF AND 1. SUPPORTS.
- FOLLOW DECK MANUFACTURERS GUIDELINES FOR PLACEMENT OF CONCRETE ON TOP 2. OF METAL FLOOR DECK. PLACE FRESH CONCRETE OVER BEAMS AND DISTRIBUTE EVENLY OVER DECK.
- HOLES LARGER THAN 6" THAT ARE TO BE CUT IN CONCRETE FLOOR DECK SHALL BE 3. REINFORCED PER TYPICAL DETAILS. HOLES LESS THAN 6" DIA MAY BE CORE DRILLED THROUGH CONCRETE FLOOR DECK AFTER OBTAINING DESIGN STRENGTH. SPACING OF HOLES DRILLED THROUGH DECK SHALL BE A MINIMUM OF THREE (3) TIME THE DIAMETER.
- METAL FLOOR DECK SHALL BE 22 GA COMPOSITE B DECK WITH 3 1/2" COMPOSITE 4 THICKNESS. ACCEPTABLE MANUFACTURERS SHALL BE VERCO, ASC, VULCRAFT, OR ENGINEER APPROVED. F_Y=50KSI.
- METAL FLOOR DECK SHALL BE FASTENED TO EACH SUPPORTING STEEL MEMBER WITH 5. HILTI X-HSN-24 FOR BASE STEEL THICKNESS 1/8" TO 3/8" AND X-ENP-19 L15 FOR STEEL THICKNESS 1/4" AND LARGER IN 36/4 PATTERN. FASTEN DECK TO SIDE SUPPORTS WITH HILTI X-HSN-24 FOR BASE STEEL THICKNESS 1/8" TO 3/8" AND X-ENP-19 L15 FOR STEEL THICKNESS 1/4" AND LARGER AT 18" C/C. DECK SEAMS SHALL BE VSC2 SIDELAP BUTTON PUNCH CONNECTIONS AT 36" C/C.
- 6. DECK SHEETS SHALL BE CONTINUOUS OVER A MINIMUM OF TWO SPANS.
- DESIGN AND PROVIDE POUR STOPS AND CLOSURES AT COMPOSITE CONCRETE DECK OPENINGS, UNO.

SEE IBC-18 FASTENING SCHEDULE (TABLE 2304.10.1) FOR GENERAL FRAMING NAILING REQUIREMENTS AND REFER TO FRAMING NAIL SCHEDULE PROVIDED FOR NAIL REQUIREMENTS.

TIMBER MATERIALS SHALL CONFORM TO THE FOLLOWING GRADES UNLESS NOTED 2.

OTHERWISE: A. WALL STUDS, TOP PLATES, BOTTOM PLATES, BEAMS, COLUMNS, AND MISCELLANEOUS LIGHT FRAMING SHALL BE DOUGLAS FIR-LARCH #2 OR BETTER. B. GLULAM BEAMS (GLB) SHALL BE 24F-V4 DF/DF.

3. WALL SHEATHING AT SHEAR WALLS SHALL BE PER PLAN.

- ROOF SHEATHING SHALL BE 5/8" APA RATED SHEATHING WITH 8d NAILING 6" C/C AT ALL 4. SUPPORTED PANEL EDGES, BLOCKING LINES, COLLECTORS, AND SHEAR WALLS AT 12" C/C AT SUPPORTS IN PANEL FIELD. NO DIAPHRAGM BLOCKING IS REQUIRED.
- 6. UNLESS NOTED OTHERWISE, ALL TIMBER HEADERS SHALL BE PER THE PROVIDED SCHEDULE.
- THE FOLLOWING CONDITIONS SHALL REQUIRE THE USE OF NATURALLY DURABLE OR 7. PRESERVATIVE TREATED WOOD IN ACCORDANCE TO IBC-18 2304.12: A. LUMBER IN DIRECT CONTACT WITH CONCRETE, MASONRY, OR SOIL.
- B. LUMBER IN MOIST OR WET ENVIRONMENTS. C. LUMBER USED IN EXTERIOR APPLICATIONS AND/OR NOT PROTECTED BY A VAPOR BARRIER.
- 11. ALL METAL FASTENERS IN CONTACT WITH NATURALLY DURABLE OR PRESERVATIVE TREATED WOOD SHALL BE GALVANIZED OR STAINLESS STEEL.
- 12. TIMBER CONNECTORS CALLED OUT BY LETTERS AND NUMBERS SHALL BE BY SIMPSON STRONG-TIE COMPANY, AS SPECIFIED IN THE LATEST EDITION OF THEIR CATALOG. PROVIDE NUMBER AND SIZE OF FASTENERS AS SPECIFIED BY THE MANUFACTURER. SUBSTITUTION OF MANUFACTURED PRODUCT IS PERMITTED WITH WRITTEN APPROVAL OF THE ENGINEER OF RECORD.
- ALL BOLTS IN WOOD MEMBERS EXCEPT AT FOUNDATION SHALL CONFORM TO ASTM A307. 13. PROVIDE WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD.
- 14. UNLESS NOTED OTHERWISE, STUD WALLS SHALL BE 2x4 AT 16 INCHES C/C AT INTERIOR WALLS AND 2x6 AT 16 INCHES C/C AT EXTERIOR WALLS.
- 15. UNLESS NOTED OTHERWISE, BOTTOM PLATES OF STRUCTURAL STUD WALLS SHALL BE CONNECTED TO SUPPORTING STRUCTURE PER THE FOLLOWING WITH F1554 GRADE 36 ANCHORS. USE A 3"x3"x1/4" PLATE WASHER.

STATEMENT OF SPECIAL INSPECTION

- IN ACCORDANCE WITH THE 2018 IBC SECTION 1704.2, THE OWNER SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PERFORM INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK AND AS SPECIFIED BELOW. CONTRACTOR SHALL COORDINATE WITH INSPECTION AND TESTING AGENCY(S) FOR REQUIRED CONSTRUCTION INSPECTIONS AND MATERIAL TESTING. SPECIAL INSPECTION REPORTS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER AND THE AUTHORITY HAVING JURISDICTION WEEKLY FOR REVIEW.
- STEEL: SPECIAL INSPECTION SHALL BE PER SECTION 1705.2 OF THE IBC AND 2. CHAPTER N OF AISC 360. SPECIAL INSPECTION OF MFRS SHALL BE IN ACCORDANCE WITH AISC 341 CHAPTER J.
- COLD FORMED STEEL DECK: SPECIAL INSPECTION PER IBC 1705.2.2 AND SD1 3.
- CONCRETE CONSTRUCTION: SPECIAL INSPECTION PER SECTION 1705.3 AND TABLE 1705.3 OF THE IBC.
- POST INSTALLED ANCHORS: SPECIAL INSPECTION SHALL BE IN ACCORDANCE WITH 8 THE ANCHORS ASSOCIATED ICC-ES ESR.
- SEISMIC RESISTANCE: SPECIAL INSPECTION FOR SEISMIC RESISTANCE SHALL BE PER SECTION 1705.12 OF THE IBC.
- A. STRUCTURAL STEEL PER AISC 341 CHAPTER J AND IBC 1705.12.1 FOR SEISMIC DESIGN CATEGORY B-F.

STATEMENT OF STRUCTURAL OBSERVATION

- IN ACCORDANCE WITH THE 2018 IBC SECTION 1704.6 STRUCTURAL OBSERVATIONS SHALL BE PERFORMED BY THE STRUCTURAL ENGINEER ON SEISMIC AND WIND RESISTING ELEMENTS OF THE STRUCTURE.
- THIS STRUCTURAL OBSERVATION IS IN ADDITION TO THE REQUIRED SPECIAL 2.
- INSPECTIONS PERFORMED BY THE SPECIAL INSPECTION AND TESTING AGENCY. CONTRACTOR SHALL INFORM THE STRUCTURAL ENGINEER 48 HOURS IN ADVANCE 3. WHEN THE CONSTRUCTION OF THE STRUCTURE IS AT A POINT TO BE OBSERVED.

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- 1. SEE **S001** FOR GENERAL NOTES.
- 2. ITEMS INDICATED TO BE REMOVED SHALL INCLUDE BELOW GRADE PORTIONS OF STRUCTURE AND FOOTING.

CALL 811 2 BUSSINESS DAYS BEFORE YOU DIG

1. SEE **S001** FOR GENERAL NOTES.

	ARCHITECTURE • ENGINEERING
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	APPROVED PMG 3/15/2022
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CALL 811 2 BUSSINESS DAYS	DWG. NO. S111 SCALE: 1/32" = 1'-0"
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SHEET NOTES

- 1. SEE **S001** FOR GENERAL NOTES.
- 2. SEE **S601** FOR SCHEDULES.
- 3. WHERE FENCE OR CHUTE POSTS CONFLICT WITH NEW FOOTINGS, CAST POSTS INTO NEW CONCRETE FOOTINGS.
- 4. SEE SURVEY FOR BENCH MARKS AND DATUMS.

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- 2. SEE **S601** FOR SCHEDULES.
- 3. WHERE FENCE OR CHUTE POSTS CONFLICT WITH NEW FOOTINGS, CAST POSTS INTO NEW CONCRETE FOOTINGS.

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SHEET NOTES

1. SEE **S001** FOR GENERAL NOTES.

2. SEE S601 FOR SCHEDULES.

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SHEET NOTES

1. SEE **S001** FOR GENERAL NOTES.

2. SEE S601 FOR SCHEDULES.

- 2x6 KICKER CONNECTED TO TOP PLATE WITH GBC CONNECTOR.
- (1) SDWC TRUSS SCREW EACH END OF TRUSS FOR TOP PLATE TO TRUSS CONNECTION.
- 3. SIMPSON CS16 STRAP AROUND CORNER INTO EACH HEADER WITH (18) 0.131X2 1/2" NAILS
- (4) A34 CLIPS EVENLY SPACED FOR TOP PLATE TO GABLE TRUSS CONNECTION.
- 2x6 SHEAR WALL WITH 7/16" SHEATHING WITH 4" EDGE NAILING AND 12" FIELD NAILING, BLOCKING REQUIRED AT PANEL EDGES.
- 6. HTT2Z HOLDOWN WITH (8) 1/4"x1 1/2" SDS.
- 2x6 SHEAR WALL WITH 7/16" SHEATHING WITH 6" EDGE NAILING AND 12" FIELD NAILING, BLOCKING RQUIRED AT PANEL EDGES.

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2. STOP GUARDRAIL AND TERMINATE WITH POST BOTH SIDES OF RAMP, IF RAMP IS SELECTED.	DA					
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- 1. SEE **S001** FOR GENERAL NOTES.
- 2. SEE S601 FOR SCHEDULES.

⟨> KEY NOTES

- 1. PROVIDE 2" MAXIMUM GAP BETWEEN STAIR AND PLATFORM GUARDRAILS.
- 2. PROVIDE 2" MAXIMUM GAP BETWEEN SCOREBOARD SUPPORT COLUMNS AND PLATFORM GUARDRAILS, BOTH SIDES OF COLUMN.

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- 1. SEE **S001** FOR GENERAL NOTES.
- 2. SEE S601 FOR SCHEDULES.

⟨> KEY NOTES

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DRAW DESIG CHEC APPR	/N SN KED OVED	MT NJ GD PM	D K DF IG	3/1 3/1 3/1 3/1	15/2 15/2 15/2	202 202 202 202	2 2 2 2
DRAW DESIG CHEC		MT NJ GC PM	D K G	3/1 3/1 3/1	15/2 15/2 15/2	202 202 202 202	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
DRAW DESIG CHEC APPR		MT NJ GE PM	1362 B ± X D	3/1 3/1 3/1 3/1			2222
DRAW DESIG CHEC APPR	N SN KED OVED	MT NJ GC PM	'A 99362	3/1 3/1 3/1			2 2 2 2 2 2
DRAW DESIG CHEC APPR		MT NJ GD PM	A, WA 99362	3/1 3/1 3/1			
DRAW DESIG CHEC APPR		MT NJ GC PM	/ALLA, WA 99362	3/1 3/1 3/1			
DRAW DESIG CHEC APPR		MT NJ GD PM	LA WALLA, WA 99362	3/1 3/1 3/1			
DRAW DESIG CHEC APPR		MT NJ GC PM	VALLA WALLA, WA 99362	3/1 3/1 3/1			
DRAW DESIG CHEC APPR			ST, WALLA WALLA, WA 99362	3/1 3/1 3/1			
DRAW DESIG CHEC APPR			\RD ST, WALLA WALLA, WA 99362	3/1 3/1 3/1			
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DRAW DESIG CHEC APPR			363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1 3/1			
DRAW DESIG CHEC APPR			363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1 3/1			
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DRAW DESIG APPR MALLA WALLA COUNTY DMA			363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1 3/1			
DRAW DESIG APPR MALLA WALLA COUNTY DMC			363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1 3/1			
DRAW DESIG APPR MALLA WALLA COUNTY DMA			363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1 3/1			
DRAW DESIG APPR APPR DWG			<u>िं</u> 5 जि. मि. 2 363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1 3/1			
DRAW DESIG APPR APPR DWG SCAL ISSUE			8 4 0 0 0 0 0 0 0 0 0	3/1 3/1 3/1 3/1 7/20			

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1. SEE **S001** FOR GENERAL NOTES.

⟨ KEY NOTES

1. TURN GUARDRAIL TO COVER GAP AND STOP 2" MAX FROM PLATFORM GUARDRAIL.

STAIR TABLE (LOWER RUN)							
STAIR #	TAIR # RISES						
STAIR 1	14	13					
STAIR 2	11	10					
STAIR 3	13	12					
STAIR 4	14	13					
STAIR 5	11	10					
STAIR 6	14	13					
STAIR 7	12	11					

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DRA	WN	AP	PF	rov //Te	VA D	L 3/*	15/2	202	22
DRA DES CHE		AP	PF N		VA >	L 3/* 3/*	15/2	202 202 202	22 22 22
DRA DES CHE APP		AP D 'ED	PF N	ROY MTE NJK	VA D = G	L 3/ ² 3/ ² 3/ ²	15/2 15/2 15/2	202 202 202 202	2 2 2 2 2
DRA DES CHE APP		AP D ÉD	PF N C	ROV MTE NJK	VA D C C	L 3/ ² 3/ ² 3/ ²	15/2 15/2 15/2	202 202 202 202	22 22 22 22
DRA DES CHE APP	AWN GIGN CKE ROV	AP D 'ED	PF N C	ROY MTE NJK	VA D C - -	L 3/ ² 3/ ² 3/ ²	15/2	202 202 202 202	22 22 22 22
DRA DES CHE APP	AWN GIGN CKE ROV					L 3/ ² 3/ ² 3/ ²	15/2	202 202 202 202	22
DRA DES CHE APP					362 3362	3/ ² 3/ ² 3/ ²	15/2		22 22 22 22
DRA DES CHE APP				ROV MTE NJK PMC	NA 99362	L 3/ ² 3/ ² 3/ ²	15/2		
DRA DES CHE APP					LLA, WA 99362	L 3/ ² 3/ ² 3/ ²			
					A WALLA, WA 99362	L 3/' 3/' 3/'			
					WALLA WALLA, WA 99362	3/ ² 3/ ² 3/ ²			
				ROV MTE NJK	D ST, WALLA WALLA, WA 99362	L 3/' 3/'			
					2HARD ST, WALLA WALLA, WA 99362	L 3/' 3/'			
					33 ORCHARD ST, WALLA WALLA, WA 99362	L 3/' 3/' 3/'			
					363 ORCHARD ST, WALLA WALLA, WA 99362	L 3/' 3/'			
					363 ORCHARD ST, WALLA WALLA, WA 99362	L 3/' 3/' 3/'			
					363 ORCHARD ST, WALLA WALLA, WA 99362	L 3/' 3/'			
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					2 363 ORCHARD ST, WALLA WALLA, WA 99362 363 ORCHARD ST, WALLA WALLA, WA 99362	L 3/ ² 3/ ²			
					363 ORCHARD ST, WALLA WALLA, WA 99362	L 3/' 3/'			
					19 1 1 1 2 363 ORCHARD ST, WALLA WALLA, WA 99362	L 3/ ² 3/ ² 3/ ²			
DRA DES CHE APP APP APP APP APP APP APP APP APP					10 10 10 20 363 ORCHARD ST, WALLA WALLA, WA 99362	L 3/ ² 3/ ² 3/ ²			

- 1. SEE **S001** FOR GENERAL NOTES.
- 2. RAMP LANDINGS TO BE TO BE SLOPED 1%.

⟨ KEY NOTES

1. FASTEN L4x4x1/4 TO RETAINING WALL WITH (4) 3/8 DIA SIMPSON TITEN HD ANCHORS AT 2'-0" C/C

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DESIG	2N	NJ	ĸ	2/4	15/2	202	2		
				3/	10/2	202			
CHEC	KED OVED	GD PM	FG	3/1 3/1	15/2	202	2		
CHEC	KED OVED	GD PM	F G	3/1	15/2	202	2		
CHEC	KED OVED	GD PM	F G	3/1	15/2	202	22		
CHEC		GD PM	F G	3/1 3/1	15/2	202	22		
CHEC		GD PM	FG	3/1 3/1	15/2	202	2		
CHEC		GD PM	99362 B 1	3/1			2 2 2 1 1		
CHEC		GD PM	WA 99362	3/1					
CHEC		GD PM	ALLA, WA 99362	3/1 3/1					
CHEC APPR			LA WALLA, WA 99362	3/1 3/1					
CHEC APPR			, WALLA WALLA, WA 99362	3/1 3/1					
CHEC APPR COUNTY			(D ST, WALLA WALLA, WA 99362	3/1 3/1					
CHEC APPR MALLA COUNTY			CHARD ST, WALLA WALLA, WA 99362	3/1 3/1					
CHEC APPR MALLA COUNTY			33 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1					
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CHEC APPR MALLA WALLA COUNTY			363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1					
CHEC APPR MALLA WALLA COUNTY			363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1					
CHEC APPR MALLA WALLA COUNTY			363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1					
CHEC APPR MALLA WALLA COUNTY DMC			363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1					
CHEC APPR MALLA WALLA COUNTY			363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1					
CHEC APPR MAILA WALLA COUNTY DMC			i di o 363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1					
CHEC APPR APPR DWG SCAL ISSUE			5 - 0 363 ORCHARD ST, WALLA WALLA, WA 99362	3/1 3/1 /20					

/2"		
3/16	SUPPORTED BEAM SIZE	NUMBER OF 3/4" DIA BOLTS
- 3/16 /	W8	2
3/8"x3 1/2" PLATE	W10	2
\downarrow	W12	3
	W14	3
	W16	4
	W18	5
3/16 ON PLAN	W21	6
DRAG	W24	7
_ LOADS	W36	10

#5	17	1 1/0	0	7 1/2	5/7	5	2 1/2
#4	19"	1 1/2"	7 1/4"	6"	1"	3"	2 1/2'
#5	24"	1 7/8"	9"	7 1/2"	1 1/4"	3 3/4"	2 1/2'
#6	35"	2 1/4"	10 3/4"	9"	2 1/4"	9"	3"
#7	58"	2 5/8"	15 3/4"	10 1/2"	2 5/8"	10 1/2"	3 1/2'
#8	76"	3"	18"	12"	3"	12"	4"

1. LENGTHS ARE BASED ON ACI 318-14 FOR f'c = 4500 psi.

2. LENGTHS ARE BASED ON CLASS B TENSION SPLICES, STAGGERING OF SPLICES IS NOT REQUIRED.

3. Lh = EMBEDMENT REQUIRED FOR TENSION HOOKS

					FOOTING SC	CHEDULE W/	PIERS				
MARK	LENGTH	WIDTH	DEPTH	TOF	REINFORCING	PIER DIM "A"	PIER DIM "B"	PIER DIM "C"	PIER REINFORCING	DETAIL/SECTION	COLUMN ANCHOR EMBED
FT1-55A	5'-0"	5'-0"	1'-0"	920'-9"	(5) #5 EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT1-55B	5'-0"	5'-0"	1'-0"	920'-3"	(5) #5 EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT1-55C	5'-0"	5'-0"	1'-0"	919'-9"	(5) #5 EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT1-55D	5'-0"	5'-0"	1'-0"	919'-3"	(5) #5 EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT1-55E	5'-0"	5'-0"	1'-0"	918'-9"	(5) #5 EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT1-66A	6'-0"	6'-0"	1'-0"	921'-3"	(6) #5 EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT1-66B	6'-0"	6'-0"	1'-0"	920'-9"	(6) #5 EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT1-66C	6'-0"	6'-0"	1'-0"	920'-3"	(6) #5 EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT1-66D	6'-0"	6'-0"	1'-0"	919'-9"	(6) #5 EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT1-66E	6'-0"	6'-0"	1'-0"	919'-3"	(6) #5 EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT1-77A	7'-0"	7'-0"	1'-0"	920'-9"	(7) #5, EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT1-77B	7'-0"	7'-0"	1'-0"	919'-3"	(7) #5, EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT1-88	8'-0"	8'-0"	1'-2"	920'-3"	(8) #5, EACH WAY, BOTTOM	-	9"	-	PIER A	DETAIL 5/S501	6" - P
FT2-55	5'-0"	5'-0"	1'-0"	922'-3"	(5) #5 EACH WAY, BOTTOM	-	-	-	-	DETAIL 6/S501	6" - F
FT2-66	6'-0"	6'-0"	1'-0"	922'-3"	(6) #5 EACH WAY, BOTTOM	-	-	-	-	DETAIL 2/S502	8" - F
FT2-77A	7'-0"	7'-0"	1'-0"	920'-9"	(7) #5, EACH WAY, TOP & BOTTOM	6"	9"	26"	PIER C	DETAIL 1/S502	8" - F
FT2-77B	7'-0"	7'-0"	1'-0"	920'-3"	(7) #5, EACH WAY, TOP & BOTTOM	6"	9"	26"	PIER C	DETAIL 1/S502	8" - F
FT2-77C	7'-0"	7'-0"	1'-0"	919'-3"	(7) #5, EACH WAY, TOP & BOTTOM	6"	9"	26"	PIER C	DETAIL 1/S502	8" - F
FT2-77D	7'-0"	7'-0"	1'-0"	918'-9"	(7) #5, EACH WAY, TOP & BOTTOM	6"	9"	26"	PIER C	DETAIL 1/S502	8" - F
FT2-88A	8'-0"	8'-0"	1'-2"	921'-3"	(8) #5, EACH WAY, TOP & BOTTOM	6"	9"	26"	PIER C	DETAIL 1/S502	8" - F
FT2-88B	8'-0"	8'-0"	1'-2"	920'-9"	(8) #5, EACH WAY, TOP & BOTTOM	6"	9"	26"	PIER C	DETAIL 1/S502	8" - F
FT2-88C	8'-0"	8'-0"	1'-2"	920'-3"	(8) #5, EACH WAY, TOP & BOTTOM	6"	9"	26"	PIER C	DETAIL 1/S502	8" - F
FT2-88D	8'-0"	8'-0"	1'-2"	919'-3"	(8) #5, EACH WAY, TOP & BOTTOM	6"	9"	26"	PIER C	DETAIL 1/S502	8" - F
FT2-88E	8'-0"	8'-0"	1'-2"	920'-3"	(8) #5, EACH WAY, TOP & BOTTOM	-	16"	-	PIER D	DETAIL 5/S501	8" - F
FT3-66	6'-0"	6'-0"	1'-0"	922'-3"	(6) #5, EACH WAY, TOP & BOTTOM	-	-	-	-	DETAIL 2/S502	8" - F
FT3-77	7'-0"	7'-0"	1'-0"	922'-3"	(7) #5, EACH WAY, TOP & BOTTOM	-	-	-	-	DETAIL 2/S502	8" - F
FT4-66	6'-0"	6'-0"	1'-0"	920'-3"	(6) #5, EACH WAY, TOP & BOTTOM	6"	9"	26"	PIER C	DETAIL 5/S501	8" - F
FT4-77	7'-0"	7'-0"	1'-0"	919'-3"	(7) #5, EACH WAY, TOP & BOTTOM		10 1/2"	-	PIER B	DETAIL 5/S501	8" - F
FT4-88	8'-0"	8'-0"	1'-2"	919'-3"	(8) #5, EACH WAY, TOP & BOTTOM		10 1/2"	-	PIER B	DETAIL 5/S501	8" - F

* F: DENOTES THE ANCHOR IS TO BE EMBED INTO THE FOOTING P: DENOTES THE ANCHOR IS TO BE EMBED INTO THE PEDESTAL

MARK	HSS BRACE SIZE	GUSSET TO BRACE WELD LENGTH	GUSSET TO BRACE WELD SIZE
BF-1	3x3x3/16	10"	3/16
BF-2	5x5x3/8	15"	5/16
BF-3	6x6x1/4	16"	1/4
BF-4	6x6x3/8	18"	5/16
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NTS

PIER D

