Meier

ADDENDUM 1

Walla Walla County Rodeo Viewing Platforms Walla Walla, WA

03/30/2022

To all Bidders:

The following changes, modifications, additions, and clarifications shall be made to the Contract Documents dated 03/16/2022 for the Walla Walla County Rodeo Viewing Platforms project. Acknowledgment of this addendum shall be indicated on your bid proposal form.

GENERAL

- 1. The pre-bid walkthrough sign in sheet is attached.
- 2. There is no plan holders list.
- 3. Only those listed on the attached pre-bid walkthrough sign in sheet are eligible to submit a bid for this project.

PRE-BID MEETING MINUTES

- Reconfirmed Sealed bids will be received at the Walla Walla County Commissioner's office, <u>on the second floor of the</u> Walla Walla County Health and Legislative Building, 314 West Main, Walla Walla, Washington 99362, until 10:00 a.m., on April 4, 2022.
- 2. The estimated construction cost for this project is between \$1,000,000 and \$2,000,000.
- 3. The note concerning hot dip galvanizing will be removed and the intent is to have all structural steel prime painted and touch upped in the field.
- 4. There will be one addendum issued on 03/30/2022.
- 5. Contractor access will be through fairgrounds Gate 9 which is accessed off S 4th Ave. The contractor will be responsible for keeping the roads clean of dirt and debris.
- 6. Contractor laydown area will be established outside of the rodeo arena within the track grounds. The contractor may also use the fairground's property outside of the track between S 4th Ave. and the track for laydown.
- 7. There are piles of fill material west of the rodeo arena that can be used by the contractor. There is no guarantee that there is an adequate amount of fill for the project.
- 8. The fairgrounds will make one of their restroom facilities, near the south end of the project, available for contractor use during construction.
- 9. Construction water will also be made available by the fairgrounds via a well.

DRAWINGS:

S001 - GENERAL NOTES: Revise dead load and notes as indicated on attached revised drawing S001.

S101 – DEMOLITION PLAN: Owner will remove existing wood framed platform, announcer's booth, and stairs. Owner will also remove structural steel beams supporting the wood framing removed.

S112 – FOUNDATION PLAN – PLATFORM A: Add keynotes for footings under stairs and revise brace frame numbers as indicated on attached revised drawing S112.

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S113 – FOUNDATION PLAN – PLATFORM B AND C: Add keynotes for footings under stairs and revise brace frame numbers as indicated on attached revised drawing S113.

S114 – FOUNDATION PLAN – PLATFORM D: Add keynote for footing under stairs and revise brace frame numbers as indicated on attached revised drawing S114.

S115 – FOUNDATION PLAN – PLATFORM E: Add keynotes for footing under stairs and revise brace frame numbers as indicated on attached revised drawing S115.

S122 – FRAMING PLAN – PLATFORM A: Revise welded wire fabric call out on slab from W1.4xW1.4 to W2.0xW2.0.

S123 – FRAMING PLAN – PLATFORM B AND C: Revise welded wire fabric call out on slab from W1.4xW1.4 to W2.0xW2.0.

S124 – FRAMING PLAN – PLATFORM D: Revise welded wire fabric call out on slab from W1.4xW1.4 to W2.0xW2.0.

S125 – FRAMING PLAN – PLATFORM E: Revise welded wire fabric call out on slab from W1.4xW1.4 to W2.0xW2.0. Revise BF-# call out at center of platform to BF-3.

S126 – ELEVATED BOOTH FRAMING PLANS: Add keynotes and call outs to booth framing plan and to brace frame elevations as indicated on attached revised drawing S126.

S401 – STAIR PLANS AND PROFILE: Add notes and detail call outs to stair support elevation as indicated on attached revised drawing S401.

S402 – RAMP PLAN AND PROFILE: Add notes and detail call outs to ramp plans and sections as indicated on attached revised drawing S402.

S501 – SECTIONS AND DETAILS: Revised base plate detail call outs as indicated on attached revised drawing S501.

S502 – SECTIONS AND DETAILS: Add details 3 thru 8 as indicated on attached revised drawing S502.

S512 – SECTIONS AND DETAILS: Add weld call out to Section J and Revise notes on Section F as indicated on attached revised drawing S512.

End of Addendum 1

PRE-BID WALKTHROUGH

Walla Walla County Rodeo Viewing Platforms Walla Walla, WA

TROV BORST Callies Welding 841-571-4176 + rov@calliesweldig.com Chaig Yeagley Fowler G4 509-308-0907 Brandon Celfourlerge.com Chaig ie Barron Welertric / WW Fair 509 525 8672 charlie@wweledic.com CoorgeBooth Booth and Sour 509 308 4338 georgeboothilie boothindsons.com Klist Chriten KCIndustriesInc 89 386 4301 KCIndustriesIncwwelgond.com Ivoy Ensures Builders Frist Source 509-540-7981 Troy. Ensure BIDR.co. LarcyDimin, Hulson Source 509-540-7981 Troy. Ensure BIDR.co. James Lund Nelson Sol 509-540-6711 james Bullon-corp.com TROP Fried Velon 509-540-6711 james Bullon-corp.com TROP Fried Velon 509-540-6711 james Bullon-corp.com TROP Fried Velon 509-540-6711 james Bullon-corp.com TROP March Drun Puri - Sol - 809-805 Jourie to sold of the source of the sold of the source of the sold
Chin Yeagley Fowler GL 509-308-0907 Brandon CE fowlerger com Charlie Barron Welertric / WW Fair 509 525 8672 charlie @ wwe /edric.com Corge Booth Booth and Som 509 308 4328 george boothis a booth and sons roo Klish CARVER KCIndustries Inc 89 386 4301 KCIndustries Incwow Cgnad.com Iroy ENSURS Builders Frist Source 504-540-7981 Troy. Ensures @ BIDR. (o. LarcyDimin, Nelson 509 520-358 larry Cruis - Carp. com TROPENSURS Builders Frist Source 504-540-7981 Troy. Ensure CBIDR. (o. LarcyDimin, Nelson 509-540-6711 james @ milson-corp. com TROPENSURS Puilter Frist Source 509-540-6711 james @ milson-corp. com TROPENSURS Puilter Puri - 509-670-1116 Purit + welding.com TARPAD HENRY PURI-7 509-981-3651 Javred Napurtur Iding.or DSN Duuc Maint 509-78-9302 Josne Start Report Veling.com TARPAD HENRY Source Sone 509-520-1604 Fron Kimbali 569 520-1604 Beau Anundson Silver Creek 541 3101974 beaution Idverse Kcuntacton
Charfie Berron Welentric / Ww Fair 509 525 8672 charlise wwelentric.com Corge Booth Booth and Some 509 308 4338 george boothinie boothind sons we Klist CARVEN KCIndustries Inc. 89 386 4301 KCIndustries Incwww.egnad.com Iroy ENSURSS Builders First Source 509-540-7981 Troy. Ensurs a C BIDR. (o. Larcy Dinis Helson Song 520-3558 lowry Cruisen - Corp. com Inco Find Nelson Song 520-3558 lowry Cruisen - Corp. com Song 520-3558 lowry Cruisen - Corp. com Tappen Dure Puri-T Song - 820-1116 purise + welding.com TARPAD HENRY PURI-7 509-987-3651 Javred happentur Iding.com OSh Duve Puri-T 509-78-9302 Joshed Strother Iding.com Dish Duve Puri-T 509-78-9302 Joshed Strother Iding.com Tappen Kimball 569 520-4604 Beau Anundson Silver Creek 541 2101974 beauties/lowree Kcontaction
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LarryDimin, Helson Song S20-3758 larry Presson-Corp. Com James Lund Nelson S09-540-6711 james Endson-Corp. Com TOPANDAN Puri-T S09-820-1116 Durit +
James fund Nelson <u>509-540-6711</u> james @ nelson-corp.com <u>TRRPAD HENRY PURI-7</u> <u>S09-820-1116</u> <u>Duri * + . welding</u> .con <u>JARPAD HENRY PURI-7</u> <u>509-987-3651</u> Javred Kapvistur Idiug.vor <u>DSh Duuc</u> <u>Puri-t</u> <u>509-78-930</u> <u>Josned Spiritive Hind</u> .co <u>Trop Kimball</u> <u>5695204604</u> <u>Begu Arundson Silver Creek</u> <u>541 3101974</u> <u>beguv@Silvercreek contractor</u>
<u>JARAAD HENRY PURI-7</u> <u>SO9-820-1116</u> <u>Durit + welding</u> .con <u>JARAAD HENRY PURI-7</u> <u>SO9-987-3651</u> <u>Javred Napuritur Iding.co</u> <u>OSH DUUC</u> <u>RUFI-7</u> <u>SO9-78-93C</u> <u>JOSHO SHOITWEETING</u> . CO <u>TOD K. wheth</u> <u>5695204604</u> <u>Begu Anundson Silver Creek</u> <u>5413101974</u> <u>begut@Silvercreekcontractor</u>
TARAAD HENRY PVRI-7 509-987-3651 Javred Hapvirturlding. un OSh Duuch Puri-t Sog-78-9362 Joshd Spiritwellind. Ca TIPS Kimball 5695204604 Begu Anundson Silver Creek 541 3101974 begur@Silvercreek contractor
Josh Duven Puri-t 509-78-930 Joshd Spirithvelding. Ca TOD Kinibali Beau Anundson Silver Creek 541 310 1974 beautosilvercreek contractor
Begu Anundson Silver Creek 541 3101974 begut@Silvercreekcontractor
Beau Arundson Silver Creek 541 310 1974 heautoSilvercreek contractor
Riley Jones 509-301-1066 Fibner @ tarragon at
Mike W. Sisemore 309-205-3750 Mike a Costraction
Mark McVane Metalcraft NN 509-876-0505 metalcraft no Qamailia
DAMAN There hay Police 509 941-1856 DAVEND RPS TIKE MO
HARON GRIMM ESF SOLUTIONS 509-730-1537 ESF SOLUTIONSHE Carlod the

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GEN	ERAL NOTES		EAF	RTHWORK	СС	NCRETE - POST INSTALLED ANCHORS
1.	ALL CONSTRUCTION SHALL BE IN ACCORDANCE WI BUILDING CODE (IBC) AND LOCAL RUI ES/STANDARI	ITH THE 2018 EDITION OF THE INTERNATIONAL DS OF GOVERNING AGENCIES HAVING	1.	REMOVE ALL VEGETATION AND COMPACT TOP 12" OF SUBGRADE UNDER FOOTINGS TO 95% OF MODIFIED PROCTOR. ASTM D1557. IF BACKFILL IS REQUIRED UNDER NEW	1.	SPECIAL INSPECTIONS ARE REQUI
~				FOOTINGS OR SLABS PROVIDE 8" MAXIMUM LIFTS OF GW TO 95% OF MODIFIED PROCTOR, ASTM D1557	2.	ANCHORS SHALL BE INSTALLED IN RECOMMENDED BY THE MANUFAC
2.	DRAWINGS.		2.	PROVIDE 6" OF 5/8" MINUS CRUSHED ROCK UNDER ALL SLABS ON GRADE COMPACTED TO 95% OF MODIFIED PROCTOR ASTM D1557.	3.	EXPANSION ANCHORS SHALL CON A. HILTI KWIK BOLT-TZ2 EXPANSIO
3.	REVIEW AFTER THE CONTRACTOR HAS REVIEWED DIMENSIONS, WEIGHTS OR GAUGES, FABRICATION COORDINATION OF THE WORK BETWEEN TRADES,	THE DOCUMENTS FOR ACCURACY. QUANTITIES, PROCESSES, CONSTRUCTION METHODS, OR CONSTRUCTION SAFETY PRECAUTIONS ARE	3.	PRIOR TO DIGGING VERIFY LOCATION AND DEPTH OF UTILITIES AND OTHER UNDERGROUND INTERFERENCES. CALL TWO BUSINESS DAYS BEFORE YOU DIG AT 811.		POWER STUD +SD2 INSTALL PI PER ESR-3037. B. EXTERIOR ANCHORS SHALL BI
Д	THE SOLE RESPONSIBILITY OF THE CONTRACTOR. DESIGN CONFORMANCE ONLY.	ENGINEER OF RECORD REVIEW IS FOR GENERAL	<u>FOL</u> 1.	JNDATIONS FOUNDATION DESIGN SHALL BE IN ACCORDANCE WITH IBC TABLE 1806.2	5.	SCREW ANCHORS SHALL CONFOR A. SIMPSON STRONG TIE TITEN H OR HILTI KH-EZ INSTALL PER E
	PURPOSE IS TO DEMONSTRATE THE WAY BY WHICH THE INFORMATION AND THE DESIGN CONCEPTS EX	H THE CONTRACTOR PROPOSES TO CONFORM TO (PRESSED IN THE CONTRACT DOCUMENTS.	2.	FOUNDATION DESIGN IS BASED ON THE ALLOWABLE SOIL PROPERTIES: SOIL BEARING 2000 PSF COEFFICIENT OF FRICTION 0.35	6.	B. ANCHORS EXPOSED TO WEAT
5.	SHOP DRAWINGS AND PRODUCT DATA THAT ARE R FABRICATION, BUT NOT LIMITED TO:		3	LATERAL BEARING PRESSURE 150 PSF/FT	0.4	
	 B. CONCRETE REINFORCING AND CAST-IN-PLAC C. STRUCTURAL STEEL SHOP DRAWINGS 	E ANCHOR DRAWINGS IF fc IS OVER 2500 PSI	0. COI	NCRETE	CA 1	ANCHORS SHALL BE ASTM E1554 G
	D. STEEL DECKING DRAWINGS AND PRODUCT D E. WOOD TRUSS SHOP DRAWINGS AND CALCUL	ATA ATIONS	1.	STRUCTURAL CONCRETE COMPRESSIVE STRENGTHS AND AIR ENTRAINMENT SHALL BE	2.	FURNISH ANCHORS PREFABRICATI
6.	CONTRACTOR IS RESPONSIBLE FOR VERIFICATION AND CONSTRUCTION CONDITIONS. CONTRACTOR S SHOP FABRICATION AND/OR FIELD ERECTION. WOR	OF SITE CONDITIONS, INSTALLATION STANDARDS SHALL FIELD VERIFY ALL DIMENSIONS PRIOR TO RK DONE WITHOUT THE ENGINEERS APPROVAL IS		AS FOLLOWS: A. FOUNDATIONS - 2,500 PSI AND NO ADDED AIR ENTRAINMENT. B. EXTERIOR SLABS ON GRADE AND PEDESTALS - 4,500 PSI AND 6% AIR-ENTRAINMENT. C. CONCRETE FLOOR DECK - 3000 PSI	3	END EMBEDDED IN CONCRETE. WA 6" MINIMUM EMBEDMENT.
7	THE RESPONSIBILITY OF THE CONTRACTOR.		2.	CAST IN PLACE CONCRETE SHALL MEET THE FOLLOWING REQUIREMENTS: ACI 117 - STANDARD SPECIFICATIONS FOR TOLERANCES FOR CONCRETE	3.	ANCHORS.
8.	THE RESPONSIBILITY OF THE GENERAL CONTRACT ALL SPECIAL INSPECTION AND TESTING SHALL BE I	PERFORMED BY AN INDEPENDENT INSPECTION		ACI 301 - SPECIFICATIONS FOR STRUCTURAL CONCRETE. ACI 302.1R - GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION.	4.	PLACE EXPANSION JOINT MATERIA EMBEDDED ITEMS BEFORE PLACIN
	AND TESTING AGENCY HIRED BY THE OWNER. CON TESTING AGENCY FOR REQUIRED CONSTRUCTION	ITRACTOR TO COORDINATE WITH INSPECTION AND INSPECTIONS AND MATERIAL TESTING.		ACI 305R - HOT WEATHER CONCRETING. ACI 306 R- COLD WEATHER CONCRETING.	5. ST	ANCHORS EXPOSED TO WEATHER
9.	THE STRUCTURAL INTEGRITY OF THIS STRUCTURE STATE. WHILE UNDER CONSTRUCTION, ALL TEMPO MAINTAIN STABILITY PRIOR TO COMPLETION SHALL	IS DESIGNED TO BE ATTAINED IN IT'S COMPLETED DRARY BRACING AND/OR SHORING REQUIRED TO BE THE RESPONSIBILITY OF THE CONTRACTOR,	3.	THE USE OF SUPER PLASTICIZERS AND WATER REDUCERS IS ALLOWED, BUT NOT REQUIRED. ALL ADMIXTURES SHALL BE CHLORIDE FREE.	1.	STRUCTURAL STEEL MATERIALS S SPECIFICATIONS:
10.	INCLUDING DESIGN AND INSTALLATION. DESIGN CRITERIA (PER 2018 IBC AND ASCE 7-16)		4.	UNLESS NOTED OTHERWISE, ALL CONCRETE FLAT WORK SHALL CONFORM TO THE FOLLOWING FINISHING TOLERANCES 1/8" GAP UNDER A 10'-0" STRAIGHT EDGE.		 A. ALL W & WT SHAPES SHALL CO B. ALL PLATES, CHANNELS, ANGL CONFORM TO A36 (36 KSI)
	14.1 RISK CATEGORY	III	Э.	WHERE CALLED FOR ON THE DRAWINGS, FLOOR SLABS ARE TO BE REINFORCED WITH WELDED WIRE FABRIC WHICH SHALL BE PLACED IN FLAT SHEETS (NOT ROLLS), AND SHALL BE LAPPED TWO FULL MESHES OF FABRIC. WELDED WIRE FABRIC (WWF) SHALL	-	C. ALL HOLLOW STRUCTURAL SE KSI) (46 FOR ROUND)
	A DEAD LOADS:	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	6	CONFORM TO ASTM A1064. Fy=60 KSI. WWW SHALL BE 6x6-W2.0X2.0.	2.	ALL DETAILING, FABRICATION, AND WITH THE REQUIREMENTS OF THE
	ROOF (FLAT) FLOOR B. LIVE LOADS (IBC 1607):	8 PSF 55 PSF	0.	SECTION A615. REINFORCING STEEL WHICH IS INDICATED ON THE PLANS AS BEING WELDED OR AS PART OF SPECIAL SHEAR WALLS OR SPECIAL MOMENT FRAMES SHALL COMPLY WITH ASTM A706 Fy=60 KSI, AND SHALL ALSO BE DEFORMED. WELDING OF	3.	FOR LATERAL FORCE RESISTING S ALL BOLTED CONNECTIONS SHALL
	ASSEMBLY AREAS STAIRS / EXITS OFFICES BOOGLING LOAD	100 PSF 100 PSF 50 PSF	7.	REINFORCING BARS SHALL BE PER AWS D1.4. ALL DETAILING, FABRICATION AND PLACEMENT OF REINFORCING STEEL SHALL COMPLY WITH THE REQUIREMENTS OF THE ACL (ACL SP 66) DETAILING MANUAL 2004	4.	OTHERWISE NOTED ON THE DRAW
	C. SNOW LOADS: GROUND SNOW LOAD	Pg=18 PSF	8.	REINFORCEMENT LAP HOOKS, ETC.; SHALL BE PER THE REINFORCEMENT TABLE UNLESS	F	HOLES MAY BE OVERSIZED BY 1/8"
	SNOW EXPOSURE FACTOR: SNOW LOAD IMPORTANCE FACTOR:	Pm=30 PSF Ce=0.9 Is=1.10	9.	THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR	5. 6	ALL WELDING SHALL BE PERFORM
	THERMAL FACTOR: SLOPE FACTOR FLAT ROOF SNOW LOAD	Ct=1.2 Cs=1.0 Pf= 30 PSF		REINFORCEMENT: A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3" B. CONCRETE EXPOSED TO EARTH OR WEATHER:	0.	PROCEDURES OF THE AMERICAN V SHALL CONFORM TO AWS D1.1:202 BEEN RE-TESTED WITHIN 6 MONTH
				#6 THROUGH #18 BARS: 2" #5 BAR, W31 OR D31 WIRE AND SMALLER: 1 1/2" C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:		WELDING FOR STRUCTURAL STEEI ELECTRODES. WELDING ON MAIN SHALL ALSO CONFORM TO AWS D1
	A. WIND DESIGN LOAD DATA. VELOCITY (3-SECGUST) EXPOSURE INTERNAL PRESSURE COFFEICIENT	V (ULT) = 110 MPH / V (ASD) = 85 MPH C 0 55		#14 AND 18 BARS: 1 1/2" #11 BAR AND SMALLER: 3/4" BEAMS, COLUMNS	7.	FIELD WELDING SYMBOLS HAVE NO IT IS THE CONTRACTOR'S RESPON
	COMPONENTS AND CLADDING DESIGN WIND WALLS 10 SF	FORCES (IBC 1603.1.4) ULTIMATE -41.16 PSF		PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS: 1 1/2" D. REINFORCING SHALL EXTEND TO THE END OF THE CONCRETE AND MAINTAIN THE COVER LISTED ABOVE AT THE ENDS.	8.	ENDS OF HOLLOW STRUCTURAL SI HAVE 3/16" CAP PLATES AND SEAL
	500 SF ROOF ZONE 1 & 2e	-33.68 PSF -26.19 PSF	11.	ALL EXPOSED CORNERS OF CONCRETE SHALL BE FORMED INTO A 3/4" x 45 DEGREE CHAMFER, OR SCRIBED WITH A CONCAVE TOOLING DEVICE UNLESS NOTED OTHERWISE.	9.	HOLES SHALL NOT BE CUT THROUG THE ENGINEER OF RECORD IN WR
	B. SEISMIC DESIGN LOAD DATA: IMPORTANCE FACTOR	1.25	12.	ACCURATELY POSITION, SUPPORT AND SECURE REINFORCEMENT.	(10.	NOT USED.
	RHO (E-W) MAPPED SPECTRAL RESPONSE ACCELERATIO	1.3 DNS:	13.	PROPORTION AND DESIGN MIXES TO RESULT IN CONCRETE SLUMP AT POINT OF PLACEMENT 4" OR 8" FOR CONCRETE WITH VERIFIED SLUMP OF 2" TO 4" BEFORE ADDING	2 11.	PRIME PAINT ALL FERROUS METAL WELDS AND SCRATCHES.
	Ss = S1 = SEISMIC SITE CLASS	0.405 0.139 D		MINUS 1 INCH. ADDITION OF WATER TO READY-MIX CONCRETE IN THE FIELD SHALL BE ALLOWED ONLY IF ON TRIP TICKET AND BEFORE DISCHARGE AND TESTING.	12.	GROUT MATERIAL FOR BASE PLATI NONMETALLIC, NON-SHRINK, PREP
	DESIGN SPECTRAL RESPONSE COEFFICIE Sds =	NTS: 0.398	14.	DEPOSIT CONCRETE IN A CONTINUOUS OPERATION UNTIL THE PLACING OF CONCRETE IS	ME	TAL FLOOR DECK
	Sd1 = SEISMIC DESIGN CATEGORY SEISMIC FORCE RESISTING SYSTEM AND F	0.215 D RESPONSE MODIFICATION FACTOR:		COMPLETE. IF THE POUR IS TO BE DISCONTINUOUS, CONTRACTOR SHALL USE CONSTRUCTION JOINTS, AS DETAILED ON THE DRAWINGS.	1.	METAL FLOOR DECK BELOW CONC SUPPORTS.
	ORDINARY CONCENTRICALLY BRACED FRAMES ORDINARY MOMENT FRAMES	R = 3.25 R = 4.5 (ANNOUNCER BOOTH FRAMING)	15.	UNLESS NOTED OTHERWISE, REINFORCING IS NOT TO EXTEND THROUGH CONSTRUCTION JOINTS OF FLOOR SLABS-ON-GRADE.	2.	FOLLOW DECK MANUFACTURERS (OF METAL FLOOR DECK. PLACE FR
	LIGHT WOOD OR COLD-FORMED STEEL FRAMED BEARING WALLS WITH WOOD SHEAR PANELS OR	``````````````````````````````````````	16.	REPAIR ALL SURFACE DEFECTS INCLUDING TIE HOLES, MINOR HONEYCOMBING AND OTHER VISUAL IRREGULARITIES WITH CEMENT MORTAR. MORTAR FOR PATCHING SHALL BE THE SAME COMPOSITION AS THAT USED IN THE CONCRETE. PATCHING SHALL BE	3.	EVENLY OVER DECK. FILL ALL CONSTRUCTION JOINTS W
	SIEEL SHEETS SEISMIC RESPONSE COEFFICIENT	к = 6.5 (ANNOUNCER BOOTH)	17.	PROVIDE (1) 4'-0" LONG #4 REBAR AT ALL RE-ENTRANT CORNERS FOR SLABS, PITS,	4.	METAL FLOOR DECK SHALL BE 22 (THICKNESS. ACCEPTABLE MANUF
	Cs DESIGN BASED SHEAR ANALYSIS PROCEDURE	0.153 0.153W	18	RECESSES, OR SLAB THICKNESS CHANGES IN THE TOP 1/3 OF THE SLAB-ON-GRADE.	5	ENGINEER APPROVED. F _Y =50KSI.
	EQUIVALENT LATERAL FORCE PER ASCE 7	-16, 12.8		GRADE AND ELEVATED DECKS. FIBERS SHALL BE EITHER POLYPROPYLENE OR NYLON. POLYPROPYLENE FIBERS SHALL BE MANUFACTURED BY THE FIBERMESH COMPANY, NYLON COMPANY, OR APPROVED EQUAL AND SHALL COMPLY WITH ASTM C1116, TYPE III. NYLON FIBERS SHALL BE MANUFACTURED BY THE NYLON COMPANY, OR ENGINEER APPROVED EQUAL.	5.	HILTI X-HSN-24 FOR BASE STEEL TH THICKNESS 1/4" AND LARGER IN 36 HILTI X-HSN-24 FOR BASE STEEL TH THICKNESS 1/4" AND LARGER AT 18 PUNCH CONNECTIONS AT 36" C/C.
					6.	DECK SHEETS SHALL BE CONTINU
					7.	DESIGN AND PROVIDE POUR STOP OPENINGS, UNO.

IRED PER IBC CHAPTER 17.

- N CONCRETE THAT IS A MINIMUM OF 21 DAYS OR AS TURER.
- IFORM TO THE FOLLOWING:
- ION ANCHORS INSTALL PER ESR-4266 OR DEWALT PER ESR-2502 OF SIMPSON STRONG TIE BOLT 2 INSTALL BE STAINLESS STEEL OR GALVANIZED.
- RM TO THE FOLLOWING:
- HD ANCHORS INSTALL PER ESR-2713 ESR-3027
- ISTALL PER ESR-3889 THER SHALL BE STAINLESS STEEL OR GALVANIZED.
- ALL NAILING REQUIREMENTS LISTED ARE BASED UPON THE USE OF COMMON WIRE NAILS (NOT SINKERS, BOX, ETC.) UNLESS NOTED OTHERWISE. ALTERNATIVE NAIL TYPES OF D PRODUCT IS NOT PERMITTED UNLESS APPROVED BY EQUIVALENT DIAMETERS MAY BE SUBSTITUTED, WITH PRIOR APPROVAL OF THE RITING. ENGINEER OF RECORD.
- GRADE 55, UNLESS NOTED OTHERWISE.
- FED WITH DOUBLE HEAVY HEX NUTS JAMMED AT THE ALL PLATE OR TRACK ANCHORS MAY BE L BOLTS WITH
- IEX NUTS FOR SECURING THE BASE PLATE TO THE
- ANCHORS IS PERMITTED. POSITION AND SECURE IN ALS, ALL CAST IN PLACE ANCHORS AND OTHER NG CONCRETE IN FORMS.
- $\sim\sim\sim\sim\sim$ R SHALL BE GALVANIZED.
- SHALL CONFORM TO THE FOLLOWING ASTM
- ONFORM TO A992 (50 KSI) ELES, BARS, M SHAPES, S SHAPES, AND FLATS SHALL
- ECTIONS (HSS) SHALL CONFORM TO A500, GRADE C (50
- ERECTION OF STRUCTURAL STEEL SHALL COMPLY E AISC 360-16 SPECIFICATION FOR STRUCTURAL STEEL MIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS SYSTEM.
- . BE MADE WITH HIGH STRENGTH BOLTS (A325) UNLESS VING. BOLTED CONNECTIONS SHALL BE "SNUG TIGHT".
- IETER + 1/16". BOLT END AND EDGE DISTANCES AND SC, UNLESS NOTED OTHERWISE. BASE PLATE BOLT
- IALL NOT BE ENLARGED BY BURNING.
- IED IN ACCORDANCE WITH THE SPECIFICATIONS AND WELDING SOCIETY BY AWS CERTIFIED WELDERS AND 20. WELDERS PERFORMING THE WORK SHALL HAVE HS PRIOR TO THE START OF STEEL FABRICATION. L SHALL BE MADE WITH E70XX LOW HYDROGEN FORCE RESISTING SYSTEM AS INDICATED ON PLANS 1.8:2016 WITH WELDERS CERTIFIED TO AWS D1.8
- IOT NECESSARILY BEEN INDICATED ON THE DRAWINGS. SIBILITY TO COORDINATE THE USE OF SHOP AND FIELD
- SECTION COLUMNS AND EXPOSED MEMBERS SHALL WELDS ALL ROUND.
- JGH BEAMS UNLESS INDICATED OR PRE-APPROVED BY ITING.
- LS FOR EXTERIOR EXPOSURE. TOUCH UP FIELDS
- TES, SLEEVES, AND EMBEDDED STEEL SHALL BE PACKAGED GROUT CONFORMING TO ASTM C 1107.

- CRETE SLABS SHALL BE ATTACHED TO ITSELF AND
- GUIDELINES FOR PLACEMENT OF CONCRETE ON TOP RESH CONCRETE OVER BEAMS AND DISTRIBUTE
- WITH SELF-LEVELING SILICONE.
- _____ GA COMPOSITE B DECK WITH 3 1/2" COMPOSITE FACTURERS SHALL BE VERCO, ASC, VULCRAFT, OR
- STENED TO EACH SUPPORTING STEEL MEMBER WITH THICKNESS 1/8" TO 3/8" AND X-ENP-19 L15 FOR STEEL 6/4 PATTERN. FASTEN DECK TO SIDE SUPPORTS WITH THICKNESS 1/8" TO 3/8" AND X-ENP-19 L15 FOR STEEL 18" C/C. DECK SEAMS SHALL BE VSC2 SIDELAP BUTTON
- JOUS OVER A MINIMUM OF TWO SPANS.
- PS AND CLOSURES AT COMPOSITE CONCRETE DECK

TIMBER

- SEE IBC-18 FASTENING SCHEDULE (TABLE 2304.10.1) FOR GENERAL FRAMING NAILING 1. REQUIREMENTS AND REFER TO FRAMING NAIL SCHEDULE PROVIDED FOR NAIL REQUIREMENTS.
- TIMBER MATERIALS SHALL CONFORM TO THE FOLLOWING GRADES UNLESS NOTED 2.
- OTHERWISE: A. WALL STUDS, TOP PLATES, BOTTOM PLATES, BEAMS, COLUMNS, AND MISCELLANEOUS LIGHT FRAMING SHALL BE DOUGLAS FIR-LARCH #2 OR BETTER. B. GLULAM BEAMS (GLB) SHALL BE 24F-V4 DF/DF.
- 3. WALL SHEATHING AT SHEAR WALLS SHALL BE PER PLAN.
- ROOF SHEATHING SHALL BE 5/8" APA RATED SHEATHING WITH 8d NAILING 6" C/C AT ALL 4. SUPPORTED PANEL EDGES, BLOCKING LINES, COLLECTORS, AND SHEAR WALLS AT 12" C/C AT SUPPORTS IN PANEL FIELD. NO DIAPHRAGM BLOCKING IS REQUIRED.
- 6. UNLESS NOTED OTHERWISE, ALL TIMBER HEADERS SHALL BE PER THE PROVIDED SCHEDULE.
- THE FOLLOWING CONDITIONS SHALL REQUIRE THE USE OF NATURALLY DURABLE OR 7. PRESERVATIVE TREATED WOOD IN ACCORDANCE TO IBC-18 2304.12: A. LUMBER IN DIRECT CONTACT WITH CONCRETE, MASONRY, OR SOIL.
- B. LUMBER IN MOIST OR WET ENVIRONMENTS. C. LUMBER USED IN EXTERIOR APPLICATIONS AND/OR NOT PROTECTED BY A VAPOR BARRIER.
- ALL METAL FASTENERS IN CONTACT WITH NATURALLY DURABLE OR PRESERVATIVE 11. TREATED WOOD SHALL BE GALVANIZED OR STAINLESS STEEL.
- 12. TIMBER CONNECTORS CALLED OUT BY LETTERS AND NUMBERS SHALL BE BY SIMPSON STRONG-TIE COMPANY, AS SPECIFIED IN THE LATEST EDITION OF THEIR CATALOG. PROVIDE NUMBER AND SIZE OF FASTENERS AS SPECIFIED BY THE MANUFACTURER. SUBSTITUTION OF MANUFACTURED PRODUCT IS PERMITTED WITH WRITTEN APPROVAL OF THE ENGINEER OF RECORD.
- ALL BOLTS IN WOOD MEMBERS EXCEPT AT FOUNDATION SHALL CONFORM TO ASTM A307. 13. PROVIDE WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD.
- UNLESS NOTED OTHERWISE, STUD WALLS SHALL BE 2x4 AT 16 INCHES C/C AT INTERIOR 14. WALLS AND 2x6 AT 16 INCHES C/C AT EXTERIOR WALLS.
- UNLESS NOTED OTHERWISE, BOTTOM PLATES OF STRUCTURAL STUD WALLS SHALL BE 15. CONNECTED TO SUPPORTING STRUCTURE PER THE FOLLOWING WITH F1554 GRADE 36 ANCHORS. USE A 3"x3"x1/4" PLATE WASHER.

STATEMENT OF SPECIAL INSPECTION

- IN ACCORDANCE WITH THE 2018 IBC SECTION 1704.2, THE OWNER SHALL EMPLOY 1. ONE OR MORE APPROVED AGENCIES TO PERFORM INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK AND AS SPECIFIED BELOW. CONTRACTOR SHALL COORDINATE WITH INSPECTION AND TESTING AGENCY(S) FOR REQUIRED CONSTRUCTION INSPECTIONS AND MATERIAL TESTING. SPECIAL INSPECTION REPORTS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER AND THE AUTHORITY HAVING JURISDICTION WEEKLY FOR REVIEW.
- STEEL: SPECIAL INSPECTION SHALL BE PER SECTION 1705.2 OF THE IBC AND CHAPTER N OF AISC 360. SPECIAL INSPECTION OF MFRS SHALL BE IN ACCORDANCE WITH AISC 341 CHAPTER J.
- COLD FORMED STEEL DECK: SPECIAL INSPECTION PER IBC 1705.2.2 AND SD1 3.
- CONCRETE CONSTRUCTION: SPECIAL INSPECTION PER SECTION 1705.3 AND TABLE 1705.3 OF THE IBC.
- POST INSTALLED ANCHORS: SPECIAL INSPECTION SHALL BE IN ACCORDANCE WITH 8. THE ANCHORS ASSOCIATED ICC-ES ESR.
- SEISMIC RESISTANCE: SPECIAL INSPECTION FOR SEISMIC RESISTANCE SHALL BE 9. PER SECTION 1705.12 OF THE IBC.
- A. STRUCTURAL STEEL PER AISC 341 CHAPTER J AND IBC 1705.12.1 FOR SEISMIC DESIGN CATEGORY B-F.

STATEMENT OF STRUCTURAL OBSERVATION

- IN ACCORDANCE WITH THE 2018 IBC SECTION 1704.6 STRUCTURAL OBSERVATIONS SHALL BE PERFORMED BY THE STRUCTURAL ENGINEER ON SEISMIC AND WIND RESISTING ELEMENTS OF THE STRUCTURE.
- THIS STRUCTURAL OBSERVATION IS IN ADDITION TO THE REQUIRED SPECIAL 2.
- INSPECTIONS PERFORMED BY THE SPECIAL INSPECTION AND TESTING AGENCY. CONTRACTOR SHALL INFORM THE STRUCTURAL ENGINEER 48 HOURS IN ADVANCE 3. WHEN THE CONSTRUCTION OF THE STRUCTURE IS AT A POINT TO BE OBSERVED.



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- 1. SEE **S001** FOR GENERAL NOTES.
- 2. SEE S601 FOR SCHEDULES.
- 3. WHERE FENCE OR CHUTE POSTS CONFLICT WITH NE FOOTINGS, CAST POSTS INTO NEW CONCRETE FOOT
- 4. SEE SURVEY FOR BENCH MARKS AND DATUMS.

⟨ KEY NOTES

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2. SEE S601 FOR SCHEDULES.						P: 50 F: 50	9.735 9.783	5.1589 3.5075
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4. SEE SURVEY FOR BENCH MARKS AND DATU	IMS.		SGN APPD					
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∑ <u>KEY NOTES</u>		3	RWNC					
 8'-0"x2'-0"x 2'-6" THICK FOOTING WITH (3) #5 I BOTTOM LONGITUDINAL AND #5 BARS AT 18 BOTTOM TRANSVERSE. 	3ARS TOP & " C/C TOP AND	$\frac{1}{2}$	DATE	3/30/22				
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- 1. SEE **S001** FOR GENERAL NOTES.
- 2. SEE S601 FOR SCHEDULES.
- 3. WHERE FENCE OR CHUTE POSTS CONFLICT WITH NEW FOOTINGS, CAST POSTS INTO NEW CONCRETE FOOTIN

- 8'-0"x2'-0"x 2'-6" THICK FOOTING WITH (3) #5 BARS TOP & BOTTOM LONGITUDINAL AND #5 BARS AT 18" C/C TOP A BOTTOM TRANSVERSE.
- 2. 2'-0"x2'-0"x 2'-0" THICK FOOTING WITH (3) #5 BARS EW, BASE PLATE PER DETAIL 1/S501 WITH 5/8" DIA HEAVY H HEAD ANCHORS WITH 6" EMBED.

	SHEET NOTES 1. SEE S001 FOR GENERAL NOTES. 2. SEE S001 FOR SCHEDULES		ARCHITECTURE • ENGINEERING 12 W. Kennewick Ave., Kennewick, WA 99336 P: 509.735.1589
	 SEE SOUTFOR SCHEDULES. WHERE FENCE OR CHUTE POSTS CONFLICT WITH NEW FOOTINGS, CAST POSTS INTO NEW CONCRETE FOOTINGS. 		F: 509.783.5075 www.meierinc.com
	 KEY NOTES 8'-0"x2'-0"x 2'-6" THICK FOOTING WITH (3) #5 BARS TOP & BOTTOM LONGITUDINAL AND #5 BARS AT 18" C/C TOP AND BOTTOM TRANSVERSE. 2'-0"x2'-0"x 2'-0" THICK FOOTING WITH (3) #5 BARS EW, T&B. BASE PLATE PER DETAIL 1/S501 WITH 5/8" DIA HEAVY HEX HEAD ANCHORS WITH 6" EMBED. 		DATE DRWN CHKD D 3/30/22 MTD GDF 1 3/30/22 MTD GDF 1
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			MALLA WALLA COUNTY RODEO VIEWING PLATFORMS 363 ORCHARD ST, WALLA WALLA, WA 99362 FOUNDATION PLAN - PLATFORM E
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1. SEE **S001** FOR GENERAL NOTES.

1. TURN GUARDRAIL TO COVER GAP AND STOP 2" MAX FROM PLATFORM GUARDRAIL.

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STAIR 6	14	13
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- 1. SEE **S001** FOR GENERAL NOTES.
- 2. RAMP LANDINGS TO BE TO BE SLOPED 1%.

⟨ KEY NOTES

 FASTEN L4x4x1/4 TO RETAINING WALL WITH (4) 3/8 DIA SIMPSON TITEN HD ANCHORS AT 2'-0" C/C

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